

### **COVER PAGE**

### Certification

It is herewith certified that this Transport Impact Assessment has been prepared according to the requirements of the South African Traffic Impact and Site Traffic Assessment Manual.



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### Application details:

a) Municipality name: Swartland Municipality

b) Type of assessment: Transport Impact Assessment

c) Particulars of the Site Development Plan: Riebeeck Kasteel, Subdivision and Rezoning Plan (Drawing No: Rev 46 by InterActive Town and Regional Planning; dated: 12 October 2023).

d) Erf numbers and farm names: Erf 878 Riebeeck-Kasteel

e) Date of report: March 2024

f) Name and address of the Assessor: Douw Louwrens B.Eng (Civil), 16 Jacobus Geldenhuys St, Onrus, 7201

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ANNEXURE A: Photographic record of existing transport facilities

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#### 1. BACKGROUND

Liezl Stodart Pr Eng was appointed to assess the transport impact for the proposed subdivision and rezoning of erf 878, Riebeeck Kasteel. The property is located on the southwestern side of the town adjacent to Church Road (MR00227), approximately 700m north of the R46 / Church Road intersection. Please see the attached *Figure 1*, Annexure B.

#### 2. DEVELOPMENT PROPOSAL

The development will entail the subdivision and rezoning of Erf 878 to include the following:

| Land Use           | GLA (m²) | Units / Erven |  |  |
|--------------------|----------|---------------|--|--|
| Single residential | -        | 60 Erven      |  |  |
| Town housing       | -        | 62 Erven      |  |  |
| Flats              | -        | 28 Units      |  |  |
| Frailcare          | -        | 60 rooms      |  |  |
| Retail             | 2661 m²  | 2             |  |  |
| Filling station    | 343 m²   | 1             |  |  |

Please see the attached **Subdivision and Rezoning Plan** (Drawing No: Rev 46 by InterActive Town and Regional Planning; dated: 12 October 2023.

It is expected that the proposed development will be completed in 2028.

### 3. STUDY AREA

The primary study area was determined as the access roads and high-order roads in the vicinity of the development. The following elements were included in the study area:

- R46 (Trunk Road 2401)
- Church Road (Main Road 227)
- Main Street (Main Road 226)
- Fontein Street
- Intersections between these roads

### 4. EXISTING TRANSPORT INFRASTRUCTURE AND ACCESS

The roads discussed below are shown in the Locality Plan (*Figure 1*), Annexure B.

The R46 (Trunk Road 2401) is a Class 2 Major Arterial. It is a single-carriageway two-way road with one lane per direction and surfaced shoulders in a rural roadside development environment.

<u>Church Road (Main Road 227)</u> is a Class 3 Minor Arterial in a semi-rural roadside development environment in the vicinity of the development. It is a single-carriageway two-way road with one lane per direction and surfaced shoulders. Paved sidewalks and on-street parking are located along a section of Church Road to the north of the Main Street intersection. Several properties also obtain direct access off Church Road along this section.

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The 2020 Provincial Access Management Guidelines document indicates an access spacing requirement of 260 m between two unsignalised full intersections and 105 m between an unsignalised full intersection (UFI) and a high-volume driveway (HVD) for Class 3 roads in a semi-rural roadside development environment.

The Erf 878 development will obtain access off Church Road via an unsignalised full intersection approximately 690 m north of the R46 / Church Road intersection. The position of the proposed access point was previously discussed with the Directorate: Road Planning, Transport and Public Works, Western Cape Government and is in line with the Swartland Municipal Spatial Development Framework (2017 – 2022), indicating the affected section of Church Road as an 'Activity Corridor'.

A left-in-only access is also proposed off Church Road approximately 100 m south of the Church Road / Main Street intersection serving the filling station and commercial component of the development.

The proposed access on Church Road will be positioned just to the south of 'The Barn' opposite an existing farm access road. The intersection will be located approximately 240 m north of the Het Vlock Kasteel access (HVD) and 190 m south of the Kloovenburg access (HVD) and proposed left-in access. Please refer to the attached *Figure 3*. The spacing therefore complies with the access spacing requirements.

<u>Main Street (Main Road 226)</u> is a Class 4 Collector Street. It is a single-carriageway two-way road with one lane per direction in an intermediate roadside development environment with an urban cross-section, surfaced sidewalks and on-street parking located along sections of the road.

<u>Fontein Street</u> is a Class 5 Local Street in a suburban roadside development environment. It is a single-carriageway two-way road with one lane per direction with multiple direct property accesses.

An unsignalised full intersection providing access on the eastern side of the development is proposed on Fontein Street approximately 150 m south of the Fontein Street / Plein Street intersection and 85m north of the Fontein Street / Van Riebeeck Street intersection.

An emergency access is proposed on the southeastern corner of the development to tie into the Fontein Street / Kloof Street intersection.

### 5. EXISTING TRAFFIC

Available traffic counts at the Main Street / Fontein Street and Fontein Street / Plein Street intersections in conjunction with peak hour traffic volumes on R46, Church Road and Main Street were used to estimate existing peak hour traffic volumes at the affected intersections.

The Main Street / Fontein Street and Fontein Street / Plein Street intersection counts were carried out on the 25<sup>th</sup> of November 2017 and traffic volumes on R46, Church Road and Main Street were obtained from the Provincial Road Network Information System (PRNIS). These surveys were conducted on the 23<sup>rd</sup> of May 2022.

The peak hour traffic volumes were adjusted with growth rates, also obtained from the PRNIS, to obtain existing 2024 peak hour traffic volumes. Details of the traffic counts are given in *Table* 1.

Table 1: Traffic count details

| Intersection          | Morning peak hour | Afternoon peak hour | % Minibus taxis | % Buses | % Heavy<br>vehicles |
|-----------------------|-------------------|---------------------|-----------------|---------|---------------------|
| R46 / Church Rd       | 07:00 - 08:00     | 17:00 – 18:00       | 1,4%            | 0,7%    | 16,8%               |
| Church Rd / Main St   | 07:00 - 08:00     | 17:00 – 18:00       | 1,3%            | 0,6%    | 13,0%               |
| Main St / Fontein St  | 09:00 - 10:00     | 14:15 – 15:15       | 0,8%            | 0,0%    | 0,5%                |
| Fontein St / Plein St | 09:00 - 10:00     | 14:15 – 15:15       | 0,0%            | 0,0%    | 0,0%                |

The affected intersections were analysed using SIDRA software. SIDRA calculates movement and intersection delays and assigns a service level based on the duration of the delay.

A level of service A denotes an excellent service level with very little delay, whereas a level of service F represents very long delays and a breakdown in service. A level of service D is generally taken as the lowest acceptable standard.

The results of the SIDRA analysis are summarised in *Table 2*. Existing traffic volumes and service levels are shown in *Figure 4*. All affected intersections currently operate at good service levels.

Table 2: Levels of service with existing (2024) traffic volumes

|                       |                  | М                        | orning pea        | k hour                   | Afternoon peak hour      |                   |                          |  |
|-----------------------|------------------|--------------------------|-------------------|--------------------------|--------------------------|-------------------|--------------------------|--|
| Intersection          | Control measure  | Inter-<br>section<br>LOS | Avg int delay (s) | Worst<br>movement<br>LOS | Inter-<br>section<br>LOS | Avg int delay (s) | Worst<br>movement<br>LOS |  |
| R46 / Church Rd       | Side-street stop | -                        | 5,3               | А                        | -                        | 5,6               | Α                        |  |
| Church Rd / Main St   | Side-street stop | -                        | 5,1               | А                        | -                        | 4,6               | В                        |  |
| Main St / Fontein St  | Side-street stop | -                        | 7,5               | Α                        | -                        | 7,5               | А                        |  |
| Fontein St / Plein St | Side-street stop | -                        | 3,6               | А                        | -                        | 4,6               | А                        |  |

### 6. BACKGROUND TRAFFIC DEMAND ESTIMATION

The TMH16 Traffic Impact Assessment Manual suggests that background traffic demand should be estimated for the design horizon and the planning horizon. The design horizon is the year when the development will be completed and the planning horizon is the year when the entire area in which the development is located, will be developed.

It is anticipated that the development will be completed by 2028 which makes this the development horizon. The site is located in a semi-rural to suburban development environment with little growth and the planning horizon was also assumed to be 2028.

Background traffic demand is constituted of two components: percentage growth and traffic build-up from other developments. An annual traffic growth rate from 2015 to 2022 of 2,53% for the R46, 0,63% for Church Road and -0,22% for Main Street was obtained from the Western Cape Government Road Network Information System (<a href="https://rnis.westerncape.gov.za/rnis/rnis\_web\_reports.main">https://rnis.westerncape.gov.za/rnis/rnis\_web\_reports.main</a>) which was used to calculate the

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increase in traffic on roads in the study area. No growth was applied to Main Street and no additional growth was applied.

The affected intersections were again analysed with the SIDRA computer program. The results are summarised in *Table 3*. The analysis shows that all intersections will continue to operate at good service levels. Year 2028 background traffic volumes and service levels are shown in *Figure 5*.

Table 3: Levels of service with Year 2028 background traffic volumes

|                       |                  | М                        | orning pea        | k hour                   | Afternoon peak hour      |                   |                          |
|-----------------------|------------------|--------------------------|-------------------|--------------------------|--------------------------|-------------------|--------------------------|
| Intersection          | Control measure  | Inter-<br>section<br>LOS | Avg int delay (s) | Worst<br>movement<br>LOS | Inter-<br>section<br>LOS | Avg int delay (s) | Worst<br>movement<br>LOS |
| R46 / Church Rd       | Side-street stop | -                        | 5,2               | А                        | -                        | 5,5               | Α                        |
| Church Rd / Main St   | Side-street stop | -                        | 5,0               | А                        | -                        | 4,6               | В                        |
| Main St / Fontein St  | Side-street stop | i                        | 7,5               | А                        | -                        | 7,5               | А                        |
| Fontein St / Plein St | Side-street stop | -                        | 3,6               | А                        | -                        | 4,6               | А                        |

#### 7. DEVELOPMENT TRIPS

Trip generation rates were obtained from the COTO TMH 17 South African Trip Data Manual and the South African Trip Generation Rate Manual (SATGRM) for the filling station.

The SATGRM document indicates a 4% calling rate at filling stations which was used. The relevant size adjustment factor, pass-by and diverted traffic percentages as indicated in the COTO TMH 17 document were calculated and used for the retail component.

The development's trip generation potential is summarised in *Table 4*.

Table 4: Development trip generation potential

|                    |             |       |      | AM   |          |       |    |     | PM   |      |      |       |     |     |
|--------------------|-------------|-------|------|------|----------|-------|----|-----|------|------|------|-------|-----|-----|
| Land Use           | GLA<br>(m²) | Units | TGR  | %IN  | %OUT     | TOTAL | IN | OUT | TGR  | %IN  | %OUT | TOTAL | IN  | оит |
| Single residential |             | 60    | 1    | 25%  | 75%      | 60    | 15 | 45  | 1    | 70%  | 30%  | 60    | 42  | 18  |
| Town housing       |             | 62    | 0.85 | 25%  | 75%      | 53    | 13 | 40  | 0.85 | 70%  | 30%  | 53    | 37  | 16  |
| Flats              |             | 28    | 0.65 | 25%  | 75%      | 18    | 5  | 14  | 0.65 | 40%  | 60%  | 18    | 7   | 11  |
| Frailcare          |             | 60    | 0.2  | 70%  | 30%      | 12    | 8  | 4   | 0.2  | 40%  | 60%  | 12    | 5   | 7   |
| Retail             | 2661        |       | 2.6  | 65%  | 35%      | 70    | 46 | 25  | 15.0 | 40%  | 60%  | 399   | 160 | 239 |
| Filling station    | 343         | 1     | 4%   | 100% | 100%     | 21    | 10 | 10  | 4%   | 100% | 100% | 35    | 18  | 18  |
|                    |             |       |      |      | Total:   | 234   | 97 | 137 |      |      |      | 577   | 268 | 309 |
|                    |             |       |      |      | Pass-by  | 21    | 10 | 10  | 88   |      | 88   | 39    | 49  |     |
|                    |             |       |      |      | Diverted | 0     | 0  | 0   | 1    |      | 116  | 46    | 69  |     |
|                    |             |       | New  | 213  | 87       | 127   |    |     |      | 373  | 183  | 191   |     |     |

The development will have the potential to generate a total of 234 trips (97 in; 137 out) during the AM peak hour and 577 trips (268 in; 309 out) during the PM peak hour. Of these, 20 trips (10 in; 10 out) during the AM peak hour and 88 trips (39 in; 49 out) during the PM peak hour,

will be pass-by trips, while 116 trips (46 in; 69 out) during the PM peak hour will be diverted trips.

Trips were distributed primarily to and from the southwest and northwest along Church Road, northeast along Main Street and directionally at the affected intersections according to existing splits experienced. The trip distribution is shown in *Figure 6*.

#### 8. TRAFFIC IMPACT

Trips generated by the proposed development were added to the background 2028 traffic volumes to obtain total traffic volumes. The affected intersections were again analysed with the increased traffic volumes to determine the traffic impact of the development. A summary of the analysis results is given in *Table 5.* Total traffic volumes and service levels are shown in *Figure 7*.

Table 5: Levels of service with total 2028 traffic volumes

|                       |                  | М                        | orning pea        | k hour                   | Afternoon peak hour      |                   |                          |
|-----------------------|------------------|--------------------------|-------------------|--------------------------|--------------------------|-------------------|--------------------------|
| Intersection          | Control measure  | Inter-<br>section<br>LOS | Avg int delay (s) | Worst<br>movement<br>LOS | Inter-<br>section<br>LOS | Avg int delay (s) | Worst<br>movement<br>LOS |
| R46 / Church Rd       | Side-street stop | i                        | 5,7               | Α                        | 1                        | 6,4               | В                        |
| Church Rd / Main St   | Side-street stop | i                        | 4,9               | А                        | 1                        | 5,0               | В                        |
| Main St / Fontein St  | Side-street stop | i                        | 7,5               | Α                        | 1                        | 7,6               | А                        |
| Fontein St / Plein St | Side-street stop | -                        | 3,2               | А                        | -                        | 4,1               | Α                        |
| Church Rd / Access    | Side-street stop | -                        | 3,0               | А                        | -                        | 5,2               | В                        |
| Fontein St / Access   | Side-street stop | -                        | 4,0               | А                        | -                        | 3,2               | А                        |

Delays at the affected intersections will only increase slightly as a result of the development by 2028. Two new access intersections will be formed which will both operate at good service levels.

All newly formed internal intersections are expected to operate at good levels of service during the AM and PM peak hours.

The development will have a moderate impact on the surrounding road network, but no improvements will be required.

### 9. ACCESS, INTERNAL ROADS AND PARKING

The proposed access off Church Rd should be designed according to the local and provincial guidelines. Attention should be given to sight distances along Church Road.

The proposed access on Fontein Street should be designed according to local guidelines. Bell-mouth radii should preferably be 6,0 m (minimum 5,0 m).

The route through the development connecting Church Road in the west with Fontein Street in the east will function as a Class 5 Local Street. The road reserve width as indicated on the attached **Subdivision & Rezoning Plan** of 13,0 m is deemed adequate. The road should have a

blacktop width of at least 6,0 m. Other internal access roads have 10,0 m wide reserves indicated which are deemed adequate. Blacktop widths of these roads are proposed to be a minimum of 5,5 m and bell-mouths should preferably be 6,0 m (minimum 5,0 m).

Parking should be provided as per the Swartland Municipality Land Use Planning By-law document. *Table 6* summarises the off-street parking requirements for the proposed development:

Table 6: Off-street parking requirements of the proposed development

| Land Use           | GLA (m²) | Units / Erven | Requirement                 | Bays required |
|--------------------|----------|---------------|-----------------------------|---------------|
| Single residential | -        | 60 Erven      | 2 bays/unit                 | 120           |
| Town housing       | -        | 62 Erven      | 2 bays/unit                 | 124           |
| Flats              | -        | 28 Units      | 1,5 bays / unit             | 42            |
| Frail care         | -        | 60 rooms      | 1 bay / 2 rooms             | 30            |
| Retail             | 2661 m²  | 2             | 6 bays / 100 m <sup>2</sup> | 160           |
| Filling station    | 343 m²   | 1             | 4 bays / 100 m <sup>2</sup> | 14            |

#### 10. PUBLIC AND NON-MOTORISED TRANSPORT

It is expected that certain portions of the development will attract public transport trips. These include the filling station and retail components. It is proposed that adequate public transport facilities should be provided on the filling station- and retail premises.

It is furthermore proposed that a surfaced sidewalk be provided along at least one side of the 13 m reserve) through the development and up to the service station premises as this is expected to be the primary pedestrian route through the development.

No further public transport or non-motorised transport facilities are proposed.

### 11. CONCLUSIONS

It can be concluded that the proposed subdivision and rezoning of Erf 878, Riebeeck Kasteel will have a moderate traffic impact. Other findings are summarised as follows:

- The application is for the subdivision and rezoning of Erf 878 to include a residential component entailing single residential, townhousing and apartments. A filling station, retail component, and frail care centre are also proposed;
- The development will have the potential to generate a total of 234 trips (97 in; 137 out) during the AM peak hour and 577 trips (268 in; 309 out) during the PM peak hour;
- The development will obtain access off Church Rd via an unsignalised full intersection approximately 690 m north of the R46 / Church Rd intersection, a left-in-only access off Church Rd approximately 100 m south of the Church Rd / Main St intersection and an unsignalised full intersection on Fontein St approximately 150 m south of the Fontein St / Plein St intersection;
- All affected intersections in the vicinity of the development currently operate at good levels of service and will continue to do so with the addition of the proposed development's peak-hour trips;

- Newly formed intersections will operate at good levels of service during the AM and PM peak hours;
- The service station and retail components of the development will attract public transport trips;
- The Class 5 Local Street (13 m reserve) through the development and up to the service station premises is expected to be the primary pedestrian route through the development.

#### 12. RECOMMENDATIONS

The recommendations made in the transport impact assessment are summarised below.

- The proposed access off Church Rd should be designed according to the local and provincial guidelines. Attention should be given to sight distances from the access along Church Road;
- The proposed access on Fontein Street should be designed according to local guidelines;
- The route through the development connecting Church Road in the west with Fontein Street in the east should have a blacktop width of at least 6,0 m. Other internal access roads should have minimum blacktop widths of 5,5 m and bell-mouth radii of 6,0m (minimum 5,0m);
- Off-street parking should be provided as per the Swartland Municipality Land Use Planning By-law document;
- It is proposed that adequate public transport facilities be provided at the filling stationand adjacent retail premises;
- It is furthermore proposed that a surfaced sidewalk be provided along at least one side of the Class 5 Local Street (13 m reserve) through the development and up to the filling station premises.

### ANNEXURE A: PHOTOGRAPHIC RECORD OF EXISTING TRANSPORT FACILITIES

### 1. R46

Photo 1a: Looking southwest along the R46 from the Church Rd intersection



Photo 1b: Looking northeast along the R46 from the Church Rd intersection



Photo 1c: Looking north along Church Rd from the R46



2. Church Road

Photo 2a: Looking north along Church Rd from the proposed access intersection



Photo 2b: Looking south along Church Rd from the proposed access intersection



Photo 2c: Looking north along Church Rd from the Main St intersection



### 3. Main Street

Photo 3a: Looking northeast along Main St from Church Rd intersection



Photo 3b: Looking northeast along Main St from Fontein St intersection



Photo 3c: Looking southwest along Main St from Fontein St intersection



4. Fontein Street

Photo 3a: Looking southeast along Fontein St from Main St intersection



Photo 3b: Looking southeast along Fontein St from proposed access intersection



Photo 3c: Looking northwest along Fontein St from proposed access intersection



### **ANNEXURE B: DRAWINGS**

| Figure | 1: | Local | lity | p | lan |
|--------|----|-------|------|---|-----|
|--------|----|-------|------|---|-----|

Figure 2: Site Development Plan

Figure 3: Access spacing plan

Figure 4: Existing 2024 traffic volumes and service levels

Figure 5: Background 2028 traffic volumes and service levels

Figure 6: Trip generation potential of proposed development

Figure 7: Total 2028 traffic volumes and service levels













